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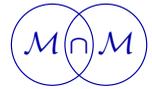
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OF THE EFFECTIVE ELASTICITY TENSORS OF MIXTURES
OF AN ELASTIC PHASE AND AN ALMOST RIGID PHASE**



TOWARDS A COMPLETE CHARACTERIZATION OF THE EFFECTIVE ELASTICITY TENSORS OF MIXTURES OF AN ELASTIC PHASE AND AN ALMOST RIGID PHASE

GRAEME W. MILTON, DAVIT HARUTYUNYAN AND MARC BRIANE

The set GU_f of possible effective elastic tensors of composites built from two materials with positive definite elasticity tensors \mathbf{C}_1 and $\mathbf{C}_2 = \delta\mathbf{C}_0$ comprising the set $U = \{\mathbf{C}_1, \delta\mathbf{C}_0\}$ and mixed in proportions f and $1 - f$ is partly characterized in the limit $\delta \rightarrow \infty$. The material with tensor \mathbf{C}_2 corresponds to a material which (for technical reasons) is almost rigid in the limit $\delta \rightarrow \infty$. This paper, and the underlying microgeometries, has many aspects in common with the companion paper “On the possible effective elasticity tensors of 2-dimensional and 3-dimensional printed materials”. The chief difference is that one has a different algebraic problem to solve: determining the subspaces of stress fields for which the thin walled structures can be rigid, rather than determining, as in the companion paper, the subspaces of strain fields for which the thin walled structure is compliant. Recalling that GU_f is completely characterized through minimums of sums of energies, involving a set of applied strains, and complementary energies, involving a set of applied stresses, we provide descriptions of microgeometries that in appropriate limits achieve the minimums in many cases. In these cases the calculation of the minimum is reduced to a finite-dimensional minimization problem that can be done numerically. Each microgeometry consists of a union of walls in appropriate directions, where the material in the wall is an appropriate p -mode material that is almost rigid to $6 - p \leq 5$ independent applied stresses, yet is compliant to any strain in the orthogonal space. Thus the walls, by themselves, can support stress with almost no deformation. The region outside the walls contains “Avellaneda material”, which is a hierarchical laminate that minimizes an appropriate sum of elastic energies.

1. Introduction

This paper is a companion to “On the possible effective elasticity tensors of 2-dimensional and 3-dimensional printed materials” [Milton et al. 2017], which gives a partial characterization of the set GU_f of effective elasticity tensors that can be

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produced in the limit $\delta \rightarrow 0$ if we mix in prescribed proportions f and $1-f$ two materials with positive definite and bounded elasticity tensors \mathbf{C}_1 and $\mathbf{C}_2 = \delta\mathbf{C}_0$. Here we consider the opposite limit $\delta \rightarrow \infty$, which corresponds to mixing in prescribed proportions an elastic phase and an almost rigid phase. Our results are summarized in the theorem in the conclusion section. For a complete introduction and summary of previous results the reader is urged to read at least the first three sections of the companion paper. The essential ideas presented here are much the same as those contained in the companion paper. However, the algebraic problem relevant to this paper, of determining when the set of walls can support a set of stress fields, is quite different from the algebraic problem encountered in the companion paper of determining when the set of walls is compliant to a set of strain fields.

The microstructures we consider involve taking three limits. First, as they have structure on multiple length scales, the homogenization limit where the ratio between length scales goes to infinity needs to be taken. Second, the limit $\delta \rightarrow \infty$ needs to be taken. Third, as the structure involves walls of width ϵ , which are very stiff to certain applied stresses, the limit $\epsilon \rightarrow 0$ needs to be taken so the contribution to the elastic energy of these walls goes to zero, when the structure is compliant to an applied strain. The limits should be taken in this order, as, for example, standard homogenization theory is justified only if δ is positive and finite, so we need to take the homogenization limit before taking the limit $\delta \rightarrow \infty$.

As in the companion paper we emphasize that our analysis is valid only for linear elasticity, and ignores nonlinear effects such as buckling, which may be important even for small deformations. It is also important to emphasize that to apply our results when phase 2 is perfectly rigid (rather than almost rigid) requires special care. Indeed, if phase 2 is perfectly rigid, then many of the microgeometries considered here do not permit the kind of motions that are permitted for any finite value of δ , no matter how large. In particular, the structures considered in Figures 6, 8, and 9(d) of the companion paper would be completely rigid if phase 2 was perfectly rigid. To permit the required motions, one has to first replace the rigid phase 2 with a composite with a small amount of phase 1 as the matrix phase, so that its effective elasticity tensor is finite but approaches infinity as the proportion of phase 1 in it tends to zero. The microgeometry in this composite needs to be much smaller than the scales in the geometries discussed here, which would involve mixtures of it and phase 1.

2. Characterizing G closures through sums of energies and complementary energies

Cherkaev and Gibiansky [1992; 1993] found that bounding sums of energies and complementary energies could lead to very useful bounds on G -closures. It was

subsequently proved in [Francfort and Milton 1994; Milton 1994] that minimums over $C_* \in GU_f$ of such sums of energies and complementary energies completely characterize GU_f in much the same way that Legendre transforms characterize convex sets: the stability under lamination of GU_f is what allows one to recover GU_f from the values of these minimums (see also Chapter 30 in [Milton 2002]). Specifically, in the case of 3-dimensional elasticity, the set GU_f is completely characterized if we know the seven “energy functions”,

$$\begin{aligned}
W_f^0(\sigma_1^0, \sigma_2^0, \sigma_3^0, \sigma_4^0, \sigma_5^0, \sigma_6^0) &= \min_{C_* \in GU_f} \sum_{j=1}^6 \sigma_j^0 : C_*^{-1} \sigma_j^0, \\
W_f^1(\sigma_1^0, \sigma_2^0, \sigma_3^0, \sigma_4^0, \sigma_5^0, \epsilon_1^0) &= \min_{C_* \in GU_f} \left[\epsilon_1^0 : C_* \epsilon_1^0 + \sum_{j=1}^5 \sigma_j^0 : C_*^{-1} \sigma_j^0 \right], \\
W_f^2(\sigma_1^0, \sigma_2^0, \sigma_3^0, \sigma_4^0, \epsilon_1^0, \epsilon_2^0) &= \min_{C_* \in GU_f} \left[\sum_{i=1}^2 \epsilon_i^0 : C_* \epsilon_i^0 + \sum_{j=1}^4 \sigma_j^0 : C_*^{-1} \sigma_j^0 \right], \\
W_f^3(\sigma_1^0, \sigma_2^0, \sigma_3^0, \epsilon_1^0, \epsilon_2^0, \epsilon_3^0) &= \min_{C_* \in GU_f} \left[\sum_{i=1}^3 \epsilon_i^0 : C_* \epsilon_i^0 + \sum_{j=1}^3 \sigma_j^0 : C_*^{-1} \sigma_j^0 \right], \quad (2-1) \\
W_f^4(\sigma_1^0, \sigma_2^0, \epsilon_1^0, \epsilon_2^0, \epsilon_3^0, \epsilon_4^0) &= \min_{C_* \in GU_f} \left[\sum_{i=1}^4 \epsilon_i^0 : C_* \epsilon_i^0 + \sum_{j=1}^2 \sigma_j^0 : C_*^{-1} \sigma_j^0 \right], \\
W_f^5(\sigma_1^0, \epsilon_1^0, \epsilon_2^0, \epsilon_3^0, \epsilon_4^0, \epsilon_5^0) &= \min_{C_* \in GU_f} \left[\left(\sum_{i=1}^5 \epsilon_i^0 : C_* \epsilon_i^0 \right) + \sigma_1^0 : C_*^{-1} \sigma_1^0 \right], \\
W_f^6(\epsilon_1^0, \epsilon_2^0, \epsilon_3^0, \epsilon_4^0, \epsilon_5^0, \epsilon_6^0) &= \min_{C_* \in GU_f} \sum_{i=1}^6 \epsilon_i^0 : C_* \epsilon_i^0.
\end{aligned}$$

In fact, Milton and Cherkaev [1995] showed it suffices to know these functions for sets of applied strains ϵ_i^0 and applied stresses σ_j^0 that are mutually orthogonal:

$$\begin{aligned}
(\epsilon_i^0, \sigma_j^0) = 0, \quad (\epsilon_i^0, \epsilon_k^0) = 0, \quad (\sigma_j^0, \sigma_\ell^0) = 0, \\
\text{for all } i, j, k, \ell \text{ with } i \neq j, i \neq k, j \neq \ell. \quad (2-2)
\end{aligned}$$

The terms appearing in the minimums have a physical significance. For example, in the expression for W_f^2 ,

$$\sum_{i=1}^2 \epsilon_i^0 : C_* \epsilon_i^0 + \sum_{j=1}^4 \sigma_j^0 : C_*^{-1} \sigma_j^0 \quad (2-3)$$

has the physical interpretation of being the sum of energies per unit volume stored in the composite with effective elasticity tensor C_* when successively subjected to

the two applied strains ϵ_1^0 and ϵ_2^0 and then to the four applied stresses $\sigma_1^0, \sigma_2^0, \sigma_3^0$, and σ_4^0 . To distinguish the terms $\epsilon_i^0 : C_* \epsilon_i^0$ and $\sigma_j^0 : C_*^{-1} \sigma_j^0$, the first is called an energy (it is really an energy per unit volume associated with the applied strain ϵ_i^0) and the second is called a complementary energy, although it too physically represents an energy per unit volume associated with the applied stress σ_j^0 .

For well-ordered materials with $C_2 \geq C_1$ (or the reverse), Avellaneda [1987] showed that there exist sequentially layered laminates of finite rank having an effective elasticity tensor $C_* = C_f^A(\epsilon_1^0, \epsilon_2^0, \epsilon_3^0, \epsilon_4^0, \epsilon_5^0, \epsilon_6^0)$ (not to be confused with the elasticity tensor $C_* = \tilde{C}_f^A(\sigma_1^0, \sigma_2^0, \sigma_3^0, \sigma_4^0, \sigma_5^0, \sigma_6^0)$ used in the companion paper) that attains the minimum in the above expression for $W_f^6(\epsilon_1^0, \epsilon_2^0, \epsilon_3^0, \epsilon_4^0, \epsilon_5^0, \epsilon_6^0)$:

$$W_f^6(\epsilon_1^0, \epsilon_2^0, \epsilon_3^0, \epsilon_4^0, \epsilon_5^0, \epsilon_6^0) = \sum_{i=1}^6 \epsilon_i^0 : C_f^A(\epsilon_1^0, \epsilon_2^0, \epsilon_3^0, \epsilon_4^0, \epsilon_5^0, \epsilon_6^0) \epsilon_i^0. \quad (2-4)$$

The effective tensor $C_* = C_f^A(\epsilon_1^0, \epsilon_2^0, \epsilon_3^0, \epsilon_4^0, \epsilon_5^0, \epsilon_6^0)$ of the Avellaneda material is found by finding a combination of the parameters entering the formula for the effective tensor of sequentially layered laminates that minimizes the sum of six elastic energies. In general this has to be done numerically, but it suffices to consider laminates of rank at most 6 if C_1 is isotropic [Francfort et al. 1995], or, using an argument of Avellaneda [1987], to consider laminates of rank at most 21 if C_1 is anisotropic (see Section 2 in the companion paper).

In the case of 2-dimensional elasticity, the set GU_f is similarly completely characterized if we know the 4 “energy functions”,

$$\begin{aligned} W_f^0(\sigma_1^0, \sigma_2^0, \sigma_3^0) &= \min_{C_* \in GU_f} \sum_{j=1}^3 \sigma_j^0 : C_*^{-1} \sigma_j^0, \\ W_f^1(\sigma_1^0, \sigma_2^0, \epsilon_1^0) &= \min_{C_* \in GU_f} \left[\epsilon_1^0 : C_* \epsilon_1^0 + \sum_{j=1}^2 \sigma_j^0 : C_*^{-1} \sigma_j^0 \right], \\ W_f^2(\sigma_1^0, \epsilon_1^0, \epsilon_2^0) &= \min_{C_* \in GU_f} \left[\left(\sum_{i=1}^2 \epsilon_i^0 : C_* \epsilon_i^0 \right) + \sigma_1^0 : C_*^{-1} \sigma_1^0 \right], \\ W_f^3(\epsilon_1^0, \epsilon_2^0, \epsilon_3^0) &= \min_{C_* \in GU_f} \sum_{i=1}^3 \epsilon_i^0 : C_* \epsilon_i^0. \end{aligned} \quad (2-5)$$

Again $W_f^3(\epsilon_1^0, \epsilon_2^0, \epsilon_3^0)$ is attained for an “Avellaneda material” consisting of a sequentially layered laminate geometry having an effective tensor $C_* = C_f^A(\epsilon_1^0, \epsilon_2^0, \epsilon_3^0)$, i.e.,

$$W_f^3(\epsilon_1^0, \epsilon_2^0, \epsilon_3^0) = \sum_{i=1}^3 \epsilon_i^0 : C_f^A(\epsilon_1^0, \epsilon_2^0, \epsilon_3^0) \epsilon_i^0. \quad (2-6)$$

The effective tensor $\mathbf{C}_* = \mathbf{C}_f^A(\boldsymbol{\epsilon}_1^0, \boldsymbol{\epsilon}_2^0, \boldsymbol{\epsilon}_3^0)$ of the Avellaneda material is found by finding a combination of the parameters entering the formula for the effective tensor of sequentially layered laminates that minimizes the sum of three elastic energies. In general this has to be done numerically, but it suffices to consider laminates of rank at most three if \mathbf{C}_1 is isotropic [Avellaneda and Milton 1989], or, using an argument of Avellaneda [1987], to consider laminates of rank at most 6 if \mathbf{C}_1 is anisotropic (see Section 2 in the companion paper).

3. Microgeometries which are associated with sharp bounds on many sums of energies and complementary energies

The analysis here of mixtures of an almost rigid phase mixed with an elastic phase is very similar to the analysis in the companion paper for mixtures of an extremely compliant phase and an elastic phase. The roles of stresses and strains are interchanged and now the challenge is to identify matrix pencils that are spanned by matrices with zero determinant, rather than symmetrized rank 1 matrices. We now have the inequalities

$$\begin{aligned}
0 &\leq W_f^0(\boldsymbol{\sigma}_1^0, \boldsymbol{\sigma}_2^0, \boldsymbol{\sigma}_3^0, \boldsymbol{\sigma}_4^0, \boldsymbol{\sigma}_5^0, \boldsymbol{\sigma}_6^0), \\
\boldsymbol{\epsilon}_1^0 : [\mathbf{C}_f^A(0, 0, 0, 0, 0, \boldsymbol{\epsilon}_1^0)]\boldsymbol{\epsilon}_1^0 &\leq W_f^1(\boldsymbol{\sigma}_1^0, \boldsymbol{\sigma}_2^0, \boldsymbol{\sigma}_3^0, \boldsymbol{\sigma}_4^0, \boldsymbol{\sigma}_5^0, \boldsymbol{\epsilon}_1^0), \\
\sum_{i=1}^2 \boldsymbol{\epsilon}_i^0 : [\mathbf{C}_f^A(0, 0, 0, 0, \boldsymbol{\epsilon}_1^0, \boldsymbol{\epsilon}_2^0)]\boldsymbol{\epsilon}_i^0 &\leq W_f^2(\boldsymbol{\sigma}_1^0, \boldsymbol{\sigma}_2^0, \boldsymbol{\sigma}_3^0, \boldsymbol{\sigma}_4^0, \boldsymbol{\epsilon}_1^0, \boldsymbol{\epsilon}_2^0), \\
\sum_{i=1}^3 \boldsymbol{\epsilon}_i^0 : [\mathbf{C}_f^A(0, 0, 0, \boldsymbol{\epsilon}_1^0, \boldsymbol{\epsilon}_2^0, \boldsymbol{\epsilon}_3^0)]\boldsymbol{\epsilon}_i^0 &\leq W_f^3(\boldsymbol{\sigma}_1^0, \boldsymbol{\sigma}_2^0, \boldsymbol{\sigma}_3^0, \boldsymbol{\epsilon}_1^0, \boldsymbol{\epsilon}_2^0, \boldsymbol{\epsilon}_3^0), \\
\sum_{i=1}^4 \boldsymbol{\epsilon}_i^0 : [\mathbf{C}_f^A(0, 0, \boldsymbol{\epsilon}_1^0, \boldsymbol{\epsilon}_2^0, \boldsymbol{\epsilon}_3^0, \boldsymbol{\epsilon}_4^0)]\boldsymbol{\epsilon}_i^0 &\leq W_f^4(\boldsymbol{\sigma}_1^0, \boldsymbol{\sigma}_2^0, \boldsymbol{\epsilon}_1^0, \boldsymbol{\epsilon}_2^0, \boldsymbol{\epsilon}_3^0, \boldsymbol{\epsilon}_4^0), \\
\sum_{i=1}^5 \boldsymbol{\epsilon}_i^0 : [\mathbf{C}_f^A(0, \boldsymbol{\epsilon}_1^0, \boldsymbol{\epsilon}_2^0, \boldsymbol{\epsilon}_3^0, \boldsymbol{\epsilon}_4^0, \boldsymbol{\epsilon}_5^0)]\boldsymbol{\epsilon}_i^0 &\leq W_f^5(\boldsymbol{\sigma}_1^0, \boldsymbol{\epsilon}_1^0, \boldsymbol{\epsilon}_2^0, \boldsymbol{\epsilon}_3^0, \boldsymbol{\epsilon}_4^0, \boldsymbol{\epsilon}_5^0).
\end{aligned} \tag{3-1}$$

The first inequality is clearly sharp, being attained when the composite consists of islands of phase 1 surrounded by a phase 2 (so that $(\mathbf{C}_*)^{-1}$ approaches 0 as $\delta \rightarrow \infty$). Again the objective is to show that many of the other inequalities are also sharp in the limit $\delta \rightarrow \infty$, at least when the spaces spanned by the applied stresses $\boldsymbol{\sigma}_j^0$ for $j = 1, 2, \dots, 6 - p$ satisfy certain properties. This space of applied stresses associated with W_f^p has dimension $6 - p$ and its orthogonal complement defines the p -dimensional space \mathcal{V}_p .

The recipe for doing this is to simply insert into a relevant Avellaneda material a microstructure occupying a thin walled region containing a p -mode material,

such that the walled structure, by itself, is very stiff when the applied stress lies in the $(6 - p)$ -dimensional subspace spanned by the σ_j^0 , yet allows strains in the orthogonal p -dimensional subspace \mathcal{V}_p spanned by the ϵ_i^0 . We say a composite with effective tensor \mathbf{C}_* built from the two materials \mathbf{C}_1 and $\mathbf{C}_2 = \delta\mathbf{C}_0$ is very stiff to a stress σ_j^0 if the complementary energy $\sigma_j^0 : \mathbf{C}_*^{-1}\sigma_j^0$ goes to zero as $\delta \rightarrow \infty$, and allows a strain ϵ_i^0 if the elastic energy $\epsilon_i^0 : \mathbf{C}_*\epsilon_i^0$ has a finite limit as $\delta \rightarrow \infty$. These p -mode materials have exactly the same construction as that specified in Section 8 of the companion paper, only now the region that was occupied by the elastic phase is now occupied by the rigid phase, and the material that was occupied by the extremely compliant phase (which becomes void in the limit $\delta \rightarrow 0$) is occupied by the elastic phase. If we happened to choose $\mathbf{C}_0 = \mathbf{C}_1$, all the moduli (and effective moduli) are simply rescaled, i.e., for any δ , and in particular for large values of δ , if a mixture of two materials with effective tensors \mathbf{C}_1 and \mathbf{C}_1/δ has effective tensor \mathbf{C}_* , then when rescaling the elasticity tensors of the two phases to $\delta\mathbf{C}_1$ and \mathbf{C}_1 , the resulting effective elasticity tensor will be $\delta\mathbf{C}_*$. Thus, the analysis of the response of the p -mode materials is essentially the same as in the companion paper. Exactly the same trial fields can be chosen to bound the response of the p -mode material. Hence we do not repeat this analysis but instead the reader is referred to Section 8 of the companion paper.

The subspace orthogonal to \mathcal{V}_p is now required to be spanned by matrices $\sigma^{(k)}$, for $k = 1, \dots, 6 - p$, such that

$$\sigma^{(k)} \mathbf{n}_k = 0 \tag{3-2}$$

for some unit vector \mathbf{n}_k . Thus the identifying feature of these matrices $\sigma^{(k)}$ is that they have zero determinant, and then \mathbf{n}_k can be chosen as a null vector of $\sigma^{(k)}$. The existence of such matrices $\sigma^{(k)}$ is proved in Section 4. The proof uses small perturbations of the applied stresses and strains. But, due the continuity of the energy functions W_f^k established in Section 5, the small perturbations do not modify the generic result. The vectors \mathbf{n}_k determine the orientation of the walls in the structure since a set of walls orthogonal to \mathbf{n} can support any stress σ such that $\sigma \mathbf{n} = 0$.

To define the thin walled structure, introduce the periodic function $H_c(x)$ with period 1 which takes the value 1 if $x - [x] \leq c$, where $[x]$ is the greatest integer less than x , and $c \in [0, 1]$ gives the relative thickness of each wall. Then for the unit vectors $\mathbf{n}_1, \mathbf{n}_2, \dots, \mathbf{n}_{6-p}$ appearing in (3-2), and for a small relative thickness $c = \epsilon$, define the characteristic functions

$$\eta_k(\mathbf{x}) = H_\epsilon(\mathbf{x} \cdot \mathbf{n}_k + k/p). \tag{3-3}$$

This characteristic function defines a series of parallel walls, as shown on the left in Figure 1, each perpendicular to the vector \mathbf{n}_j , where $\eta_j(\mathbf{x}) = 1$ in the wall material. The additional shift term k/p in (3-3) ensures the walls associated with

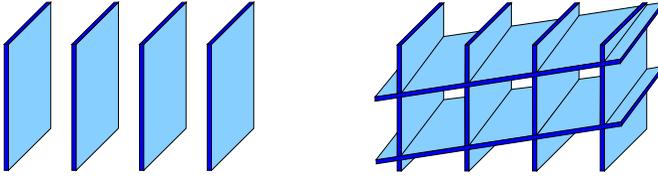


Figure 1. Example of walled structures. On the left we have a “rank 1” walled structure and on the right a “rank 2” walled structure. The generalization to walled structures of any rank is obvious, and precisely defined by the characteristic function (3-4) that is 0 in the walls, and 1 in the remaining material.

k_1 and k_2 do not intersect when it happens that $\mathbf{n}_{k_1} = \mathbf{n}_{k_2}$, at least when ϵ is small. We emphasize that ϵ is not a homogenization parameter, but rather represents a volume fraction of walls.

Now define the characteristic function

$$\chi_*(\mathbf{x}) = \prod_{k=1}^p (1 - \eta_k(\mathbf{x})). \quad (3-4)$$

If $p \leq 3$, this is usually a periodic function of \mathbf{x} , an exception being if $p = 3$ and there are no nonzero integers z_1, z_2 , and z_3 such that $z_1 \mathbf{n}_1 + z_2 \mathbf{n}_2 + z_3 \mathbf{n}_3 = \mathbf{0}$. More generally, $\chi_*(\mathbf{x})$ is a quasiperiodic function of \mathbf{x} . The walled structure is where $\chi_*(\mathbf{x})$ takes the value 0. In the case $p = 2$ the walled structure is illustrated on the right in Figure 1.

The walled structure is where $\chi_*(\mathbf{x})$ given by (3-4) takes the value 0. Inside it we put a p -mode material with effective tensor $\mathbf{C}_*^2 = \mathbf{C}_*(\mathcal{V}_p)$ that allows any applied strain $\boldsymbol{\epsilon}^0$ in the space \mathcal{V}_p but which is very stiff to any stress $\boldsymbol{\sigma}^0$ orthogonal to the space \mathcal{V}_p . Using the six matrices

$$\mathbf{v}_1 = \boldsymbol{\sigma}_1^0 / |\boldsymbol{\sigma}_1^0|, \dots, \mathbf{v}_{6-p} = \boldsymbol{\sigma}_{6-p}^0 / |\boldsymbol{\sigma}_{6-p}^0|, \mathbf{v}_{7-p} = \boldsymbol{\epsilon}_1^0 / |\boldsymbol{\epsilon}_1^0|, \dots, \mathbf{v}_6 = \boldsymbol{\epsilon}_p^0 / |\boldsymbol{\epsilon}_p^0| \quad (3-5)$$

as our basis for the 6-dimensional space of 3×3 symmetric matrices, the compliance tensor $[\mathbf{C}_*(\mathcal{V}_p)]^{-1}$ in this basis takes the limiting form

$$\lim_{\delta \rightarrow \infty} [\mathbf{C}_*(\mathcal{V}_p)]^{-1} = \begin{pmatrix} 0 & 0 \\ 0 & \mathbf{B} \end{pmatrix}, \quad (3-6)$$

where \mathbf{B} represents a (strictly) positive definite $p \times p$ matrix and the 0 on the diagonal represents the $(6-p) \times (6-p)$ zero matrix. Inside the walled structure, where $\chi_*(\mathbf{x}) = 1$, we put the Avellaneda material with effective elasticity tensor

$$\mathbf{C}_*^1 = \mathbf{C}_f^A(0, \dots, 0, \boldsymbol{\epsilon}_1^0, \dots, \boldsymbol{\epsilon}_p^0).$$

In a variational principle similar to (4-4) in the companion paper (i.e., treating the Avellaneda material and the p -mode material both as homogeneous materials with effective tensors $\mathbf{C}_*^1 = \mathbf{C}_f^A$ and $\mathbf{C}_*^2 = \mathbf{C}_*(\mathcal{V}_p)$, respectively) we choose trial strain fields that are constant,

$$\underline{\epsilon}_i(\mathbf{x}) = \epsilon_i^0, \quad \text{for } i = 1, 2, \dots, p, \quad (3-7)$$

thus trivially fulfilling the differential constraints, and trial stress fields of the form

$$\underline{\sigma}_j(\mathbf{x}) = \sum_{k=1}^{6-p} \sigma_{j,k} \eta_k(\mathbf{x}) / \epsilon, \quad (3-8)$$

which are required to have the average values

$$\sigma_j^0 = \langle \underline{\sigma}_j \rangle = \sum_{k=1}^{6-p} \sigma_{j,k}, \quad (3-9)$$

and the matrices $\sigma_{i,j}$ are additionally required to lie in the space orthogonal to \mathcal{V}_p (so they cost very little energy) and satisfy

$$\sigma_{j,k} = c_{j,k} \boldsymbol{\sigma}^{(k)}, \quad (3-10)$$

for some choice of parameters $c_{j,k}$ to ensure that $\sigma_{j,k} \mathbf{n}_k = 0$ and hence that $\underline{\sigma}_j(\mathbf{x})$ satisfies the differential constraints of a stress field—this requires $\underline{\sigma}_j(\mathbf{x}) \mathbf{n}_k$ to be continuous across any interface with normal \mathbf{n}_k . Additionally, the $c_{j,k}$ in (3-10) should be chosen so the σ_j^0 given by (3-9) are orthogonal.

To find upper bounds on the energy associated with this trial stress field, first consider those parts of the walled structure that are outside of any junction regions, i.e., where for some k we have $\eta_k(\mathbf{x}) = 1$, while $\eta_s(\mathbf{x}) = 0$ for all $s \neq k$. An upper bound for the volume fraction occupied by the region where $\eta_k(\mathbf{x}) = 1$ while $\eta_s(\mathbf{x}) = 0$ for all $s \neq k$ is of course ϵ , as this represents the volume of the region where $\eta_k(\mathbf{x}) = 1$. The associated energy per unit volume of the trial stress field in those parts of the walled structure that are outside of any junction regions is bounded above by

$$\sum_{k=1}^{6-p} \sigma_{j,k} : [\mathbf{C}_*(\mathcal{V}_p)]^{-1} \sigma_{j,k} / \epsilon. \quad (3-11)$$

With an appropriate choice of multimode material, one can construct bounded trial stress fields that are essentially concentrated in phase 2, and consequently, $\sigma_{j,k} : [\mathbf{C}_*(\mathcal{V}_p)]^{-1} \sigma_{j,k}$ is bounded above by a quantity proportional to $1/\delta$. Our assumption that we take the limit $\delta \rightarrow \infty$ before taking the limit $\epsilon \rightarrow 0$ ensures that $1/(\delta\epsilon) \rightarrow 0$, and thus ensures that the quantity (3-11) goes to zero in this limit.

Next, consider those junction regions where only two walls meet, i.e., where for some k_1 and $k_2 > k_1$, \mathbf{x} is such that $\eta_{k_1}(\mathbf{x}) = \eta_{k_2}(\mathbf{x}) = 1$ while $\eta_s(\mathbf{x}) = 0$ for all s not equal to k_1 or k_2 . Provided $\mathbf{n}_{k_1} \neq \mathbf{n}_{k_2}$, an upper bound for the volume fraction occupied by each such junction region is ϵ^2 . Then the associated energy per unit volume of the trial stress field in these junction regions where only two walls meet is bounded above by

$$\sum_{k_1=1}^{6-p} \sum_{k_2=k_1+1}^{6-p} (\boldsymbol{\sigma}_{i,k_1} + \boldsymbol{\sigma}_{i,k_2}) : [\mathbf{C}_*(\mathcal{V}_p)]^{-1} (\boldsymbol{\sigma}_{j,k_1} + \boldsymbol{\sigma}_{j,k_2}). \quad (3-12)$$

Thus, the powers of ϵ cancel and this energy density goes to zero if the multimode material is easily compliant to the strains $\boldsymbol{\sigma}_{j,k_1} + \boldsymbol{\sigma}_{j,k_2}$ for all k_1 and k_2 with $k_2 > k_1$.

Finally, consider those junction regions where three or more walls meet, i.e., for some $k_1, k_2 > k_1$, and $k_3 > k_2$, \mathbf{x} is such that $\eta_{k_i}(\mathbf{x}) = 1$ for $i = 1, 2, 3$. For a given choice of $k_1, k_2 > k_1$, and $k_3 > k_2$ such that the three vectors $\mathbf{n}_{k_1}, \mathbf{n}_{k_2}$, and \mathbf{n}_{k_3} are not coplanar, an upper bound for the volume fraction occupied by this region is ϵ^3 . In the case that the three vectors $\mathbf{n}_{k_1}, \mathbf{n}_{k_2}$, and \mathbf{n}_{k_3} are coplanar, we can ensure that the volume fraction occupied by this region is ϵ^3 or less by appropriately translating one or two walled structures, i.e., by replacing $\eta_{k_m}(\mathbf{x})$ with $\eta_{k_m}(\mathbf{x} + \alpha_i \mathbf{n}_{k_m})$ for $m = 2, 3$, for an appropriate choice of α_2 and α_3 between 0 and 1. Since the energy density of the trial field in these regions scales as $\epsilon^3/\epsilon^2 = \epsilon$, we can ignore this contribution in the limit $\epsilon \rightarrow 0$ as it goes to zero too.

From this analysis of the energy densities associated with the trial fields it follows that one does not necessarily need the pentamode, quadramode, trimode, bimode, and unimode materials as appropriate for the material inside the walled structure. Instead, by modifying the construction, it suffices to use only pentamode and quadramode materials. In the walled structure we now put pentamode materials in those sections where for some k , we have $\eta_k(\mathbf{x}) = 1$ while $\eta_{k'}(\mathbf{x}) = 0$ for all $k' \neq k$. Each pentamode material is very stiff to the single stress $\boldsymbol{\sigma}^{(k)}$ appropriate to the wall under consideration. In each junction region of the walled structure where $\eta_{k_1}(\mathbf{x}) = \eta_{k_2}(\mathbf{x}) = 1$ for some $k_1 \neq k_2$ while $\eta_k(\mathbf{x}) = 0$ for all k not equal to k_1 or k_2 , we put a quadramode material which is very stiff to any stress in the subspace spanned by $\boldsymbol{\sigma}^{(k_1)}$ and $\boldsymbol{\sigma}^{(k_2)}$ as appropriate to the junction region under consideration. In the remaining junction regions of the walled structure (where three or more walls intersect) we put phase 1. The contribution to the average energy of the fields in these regions vanishes as $\epsilon \rightarrow 0$ as discussed above.

By these constructions we effectively obtain materials with elasticity tensors \mathbf{C}_* such that

$$\lim_{\delta \rightarrow \infty} (\mathbf{C}_*)^{-1} = \Pi_p (\mathbf{C}_f^A)^{-1} \Pi_p, \quad (3-13)$$

where \mathbf{I} is the fourth-order identity matrix, Π_p is the fourth-order tensor that is the

projection onto the space \mathcal{V}_p , and C_f^A is the relevant Avellaneda material. In the basis (3-5), Π_p is represented by the 6×6 matrix that has the block form

$$\Pi_p = \begin{pmatrix} 0 & 0 \\ 0 & I_p \end{pmatrix}, \quad (3-14)$$

where I_p represents the $p \times p$ identity matrix and the 0 on the diagonal represents the $(6 - p) \times (6 - p)$ zero matrix.

In the case $d = 2$ the analysis simplifies in the obvious way. We have the inequalities

$$\begin{aligned} 0 &\leq W_f^0(\sigma_1^0, \sigma_2^0, \sigma_3^0), \\ \epsilon_1^0 : [C_f^A(0, 0, \epsilon_1^0)]\epsilon_1^0 &\leq W_f^1(\sigma_1^0, \sigma_2^0, \epsilon_1^0), \\ \sum_{i=1}^2 \epsilon_i^0 : C_f^A(0, \epsilon_1^0, \epsilon_2^0)\epsilon_i^0 &\leq W_f^2(\sigma_1^0, \epsilon_1^0, \epsilon_2^0), \end{aligned} \quad (3-15)$$

the first one of which is sharp in the limit $\delta \rightarrow \infty$ being attained when the material consists of islands of phase 1 surrounded by phase 2. The recipe for showing that the bound (3-15) on $W_f^1(\sigma_1^0, \sigma_2^0, \epsilon_1^0)$ is sharp for certain values of σ_1^0 and σ_2^0 and that the bound (2-5) on $W_f^2(\sigma_1^0, \epsilon_1^0, \epsilon_2^0)$ is sharp for certain values of σ_1^0 is almost exactly the same as in the 3-dimensional case: insert into the Avellaneda material a thin walled structure of unimode and bimode materials, respectively, so that it is very stiff to any stress in the space spanned by σ_1^0 and σ_2^0 in the case of W_f^1 , or so that it is very stiff to the stress σ_1^0 in the case of W_f^2 .

4. The algebraic problem: characterizing those symmetric matrix pencils spanned by zero determinant matrices

Now we are interested in the following question: *Given k linearly independent symmetric $d \times d$ matrices A_1, A_2, \dots, A_k , find necessary and sufficient conditions such that there exists linearly independent matrices $\{B_i\}_{i=1}^k$ spanned by the basis elements A_i such that $\det(B_i) = 0$. It is assumed that $d = 2$ or 3 and $1 \leq k \leq k_d$, where $k_2 = 2$ and $k_3 = 5$. Here we are working in the generic situation, i.e., we prove the algebraic result for a dense set of matrices. The continuity result of Section 5 will allow us to conclude for the whole set of matrices. Actually, the proof below also shows that the algebraic result holds for the complement of a zero measure set of matrices.*

Theorem 4.1. *The above problem is solvable if and only if the matrices A_i for $i = 1, \dots, k$ satisfy the following conditions:*

$$(i) \quad \det(A_1) = 0, \quad \text{if } k = 1, \quad d = 2, 3. \quad (4-1)$$

$$(ii) \quad (\alpha_1\gamma_2 + \alpha_2\gamma_1 - 2\beta_1\beta_2)^2 > 4 \det(\mathbf{A}_1) \det(\mathbf{A}_2), \quad \text{if } k = d = 2, \quad (4-2)$$

where

$$\mathbf{A}_i = \begin{pmatrix} \alpha_i & \beta_i \\ \beta_i & \gamma_i \end{pmatrix}. \quad (4-3)$$

$$(iii) \quad \Delta = 18 \det(\mathbf{A}_1) \det(\mathbf{A}_2) S_1 S_2 - 4S_1^3 \det(\mathbf{A}_2) + S_1^2 S_2^2 - 4S_2^3 \det(\mathbf{A}_1) \\ - 27 \det(\mathbf{A}_1)^2 \det(\mathbf{A}_2)^2 > 0, \quad \text{if } k = 2, d = 3, \quad (4-4)$$

where $S_i = \sum_{j=1}^3 s_{ij}$ for $i = 1, 2$ and s_{ij} is the determinant of the matrix obtained by replacing the j -th row of \mathbf{A}_i by the j -th row of \mathbf{A}_{i+1} , where by convention we have $\mathbf{A}_3 = \mathbf{A}_1$.

$$(iv) \quad \text{Always solvable if } k \geq 3, d = 3. \quad (4-5)$$

Remark. In fact, the condition (4-1), that $\det(\mathbf{A}_1) = 0$, could be excluded since we are considering the generic case. It is inserted because we can treat it explicitly.

Proof. We consider all the cases separately.

Case (i): $k = 1$. In this case one must obviously have $\det(\mathbf{A}_1) = 0$.

Case (ii): $k = 2, d = 2$. We can without loss of generality assume that (by small perturbations) $\det(\mathbf{A}_i) \neq 0$ for $i = 1, 2$. For $\eta, \mu \in \mathbb{R}^2$, denote $\mathbf{A}(\eta, \mu) = \eta \mathbf{A}_1 + \mu \mathbf{A}_2$, and thus for the equality

$$\det(\mathbf{A}(\eta, \mu)) = \det(\mathbf{A}_1)\eta^2 + (\alpha_1\gamma_2 + \alpha_2\gamma_1 - 2\beta_1\beta_2)\eta\mu + \det(\mathbf{A}_2)\mu^2 \quad (4-6)$$

to happen, one must first of all have $\mu \neq 0$; thus, dividing by μ^2 and setting $t = \eta/\mu$, we get that the quadratic equation

$$\frac{1}{\mu^2} \det(\mathbf{A}(\eta, \mu)) = \det(\mathbf{A}_1)t^2 + (\alpha_1\gamma_2 + \alpha_2\gamma_1 - 2\beta_1\beta_2)t + \det(\mathbf{A}_2) = 0 \quad (4-7)$$

must have two different solutions, i.e., the discriminant is strictly positive, which amounts to exactly (4-2).

Case (iii): $k = 2, d = 3$. Again, we can without loss of generality assume that $\det(\mathbf{A}_i) \neq 0$ for $i = 1, 2$. Set again $\mathbf{A}(\eta, \mu) = \eta \mathbf{A}_1 + \mu \mathbf{A}_2$; thus we must have that the equation

$$\det(\mathbf{A}(\eta, \mu)) = \det(\mathbf{A}_1)\eta^3 + S_1\eta^2\mu + S_2\eta\mu^2 + \det(\mathbf{A}_2)\mu^3 = 0 \quad (4-8)$$

has at least two different real roots, which by Cardan's condition gives

$$\Delta = 18 \det(\mathbf{A}_1) \det(\mathbf{A}_2) S_1 S_2 - 4S_1^3 \det(\mathbf{A}_2) + S_1^2 S_2^2 \\ - 4S_2^3 \det(\mathbf{A}_1) - 27 \det(\mathbf{A}_1)^2 \det(\mathbf{A}_2)^2 > 0, \quad (4-9)$$

which is exactly (4-4).

Case (iv): $k \geq 3$, $d = 3$. Let us consider the case $k = 3$ first. Let us show that we can assume, without loss of generality, that $\det(\mathbf{A}_1) = \det(\mathbf{A}_2) = 0$, by proving that there exist numbers $\eta_i \neq 0$ for $i = 1, 2$ such that the matrices $\mathbf{B}_1 = \eta_1 \mathbf{A}_1 + \mathbf{A}_2$ and $\mathbf{B}_2 = \eta_2 \mathbf{A}_1 + \mathbf{A}_3$ have zero determinant. Indeed, we assume without loss of generality that $\det(\mathbf{A}_i) \neq 0$ for $i = 1, 2, 3$. We would then like to have

$$\det(\mathbf{B}_1)(\eta_1) = \eta_1^3 \det(\mathbf{A}_1) + \eta_1^2(\cdot) + \eta_1(\cdot) + \det(\mathbf{A}_2) = 0, \quad (4-10)$$

which has a nonzero root η_1 , being a cubic equation with $\det(\mathbf{B}_1)(0) = \det(\mathbf{A}_2) \neq 0$. Similarly, the equation $\det(\mathbf{B}_2)(\eta_2) = 0$ has a nonzero solution η_2 . The matrices \mathbf{B}_1 , \mathbf{B}_2 , and \mathbf{A}_1 are linearly independent, because the linear independence of \mathbf{B}_1 , \mathbf{B}_2 , and \mathbf{A}_1 is equivalent to the condition

$$\det \begin{pmatrix} \eta_1 & 1 & 0 \\ \eta_2 & 0 & 1 \\ 1 & 0 & 0 \end{pmatrix} = 1 \neq 0. \quad (4-11)$$

Assume now that \mathbf{A}_1 , \mathbf{A}_2 , and \mathbf{A}_3 are linearly independent and

$$\det(\mathbf{A}_1) = \det(\mathbf{A}_2) = 0. \quad (4-12)$$

For any $\eta, \mu \in \mathbb{R}$, consider the matrix

$$\mathbf{B}_3 = \mathbf{B}(\eta, \mu) = \mathbf{A}_3 + \eta \mathbf{A}_1 + \mu \mathbf{A}_2.$$

It is clear that the triple $\mathbf{A}_1, \mathbf{A}_2, \mathbf{B}_3$ is linearly independent, so we would like to show that there exist $\eta, \mu \in \mathbb{R}$, such that $\det(\mathbf{B}_3) = 0$. Assume, by contradiction, that

$$\det(\mathbf{B}_3) \neq 0, \quad \text{for all } \eta, \mu \in \mathbb{R}. \quad (4-13)$$

Let us then show that the condition (4-13) implies that $c_1 = c_2 = 0$, where, taking into account the condition (4-12), we have that

$$\det(\mathbf{B}_3) = c_1 \eta^2 \mu + c_2 \eta \mu^2 + c_3 \eta \mu + c_4 \eta^2 + c_5 \mu^2 + c_6 \eta + c_7 \mu + \det(\mathbf{A}_3). \quad (4-14)$$

Indeed, if $c_1 \neq 0$, then taking $\eta = \mu^2$ we get that the equation $\det(\mathbf{B}(\mu^2, \mu)) = 0$ would have a solution $\mu \in \mathbb{R}$, being a fifth-order equation; thus, we get $c_1 = c_2 = 0$. Next, by perturbing the elements of \mathbf{A}_1 and \mathbf{A}_2 if necessary, we can reach the situation where no entries and second-order minors of both \mathbf{A}_1 and \mathbf{A}_2 vanish, by first reaching the situation where \mathbf{A}_1 and \mathbf{A}_2 have no zero entries. If we now perturb any ij and ik elements of \mathbf{A}_1 by small numbers ϵ and δ , where $j \neq k$, then to keep the condition $\det(\mathbf{A}_1) = 0$, we must have ϵ and δ satisfying

$$\epsilon \cdot \text{cof}_{ij}(\mathbf{A}_1) + \delta \cdot \text{cof}_{ik}(\mathbf{A}_1) = 0. \quad (4-15)$$

On the other hand, the condition $c_2 = 0$ must not be violated by that perturbation, thus we must have as well

$$\epsilon \cdot \text{cof}_{ij}(\mathbf{A}_2) + \delta \cdot \text{cof}_{ik}(\mathbf{A}_2) = 0. \quad (4-16)$$

The last two conditions then imply that the cofactor matrix $\text{cof } \mathbf{A}_1$ is a multiple of the cofactor matrix $\text{cof } \mathbf{A}_2$, i.e.,

$$\text{cof}(\mathbf{A}_2) = t \cdot \text{cof}(\mathbf{A}_1), \quad t \neq 0. \quad (4-17)$$

Again, a small perturbation of the 11 and 12 elements of \mathbf{A}_1 by ϵ and δ satisfying (4-15) with $i = j = 1, k = 2$ does not violate the condition $\det(\mathbf{A}_1) = 0$, thus it must not violate the condition (4-16). Observe that the above perturbation does not change the cofactor $\text{cof}_{11}(\mathbf{A}_1)$, but it changes the cofactor element $\text{cof}_{33}(\mathbf{A}_1)$, which means that the desired condition $\det(\mathbf{B}_3) = 0$ can be reached by small perturbations. The case $k = d = 3$ is now done.

Assume now $k \geq 4$ and $d = 3$. By the previous step, in the space spanned by $\mathbf{A}_1, \mathbf{A}_2$, and \mathbf{A}_3 there are three matrices $\mathbf{A}'_1, \mathbf{A}'_2$, and $\mathbf{B}_3 = \mathbf{A}_3 + \eta_3 \mathbf{A}'_1 + \mu_3 \mathbf{A}'_2$ that are linearly independent with zero determinant. Then, again by the previous step, we can find linearly independent matrices $\mathbf{B}_1, \dots, \mathbf{B}_k$ that have the form $\mathbf{B}_1 = \mathbf{A}'_1, \mathbf{B}_2 = \mathbf{A}'_2$, and $\mathbf{B}_i = \mathbf{A}_i + \eta_i \mathbf{A}'_1 + \mu_i \mathbf{A}'_2$ for $3 \leq i \leq k$ and that are linearly independent and have zero determinant. \square

5. Continuity of the energy functions

It follows from the preceding analysis that we can determine the three energy functions

$$\begin{aligned} W_f^1(\boldsymbol{\sigma}_1^0, \boldsymbol{\sigma}_2^0, \boldsymbol{\sigma}_3^0, \boldsymbol{\sigma}_4^0, \boldsymbol{\sigma}_5^0, \boldsymbol{\epsilon}_1^0), \\ W_f^2(\boldsymbol{\sigma}_1^0, \boldsymbol{\sigma}_2^0, \boldsymbol{\sigma}_3^0, \boldsymbol{\sigma}_4^0, \boldsymbol{\epsilon}_1^0, \boldsymbol{\epsilon}_2^0), \\ W_f^3(\boldsymbol{\sigma}_1^0, \boldsymbol{\sigma}_2^0, \boldsymbol{\sigma}_3^0, \boldsymbol{\epsilon}_1^0, \boldsymbol{\epsilon}_2^0, \boldsymbol{\epsilon}_3^0) \end{aligned}$$

in the limit $\delta \rightarrow \infty$ for almost all combinations of applied fields. Here we establish that these energy functions are continuous functions of the applied fields in the limit $\delta \rightarrow \infty$, and therefore we obtain expressions for the energy functions for all combinations of applied fields in this limit.

Recall that the set GU_f is characterized by its W -transform. For example, part of it is described by the function

$$W_f^2(\boldsymbol{\sigma}_1^0, \boldsymbol{\sigma}_2^0, \boldsymbol{\sigma}_3^0, \boldsymbol{\sigma}_4^0, \boldsymbol{\epsilon}_1^0, \boldsymbol{\epsilon}_2^0) = \min_{\mathbf{C}_* \in GU_f} \left[\sum_{i=1}^2 \boldsymbol{\epsilon}_i^0 : \mathbf{C}_* \boldsymbol{\epsilon}_i^0 + \sum_{j=1}^4 \boldsymbol{\sigma}_j^0 : \mathbf{C}_*^{-1} \boldsymbol{\sigma}_j^0 \right]. \quad (5-1)$$

Here we want to show that such energy functions are continuous in their arguments. Let the compliance tensor $[\mathbf{C}_*(\boldsymbol{\sigma}_1^0, \boldsymbol{\sigma}_2^0, \boldsymbol{\sigma}_3^0, \boldsymbol{\sigma}_4^0, \boldsymbol{\epsilon}_1^0, \boldsymbol{\epsilon}_2^0)]^{-1}$ be a minimizer of (5-1),

and suppose we perturb the applied stress fields σ_j^0 by $\delta\sigma_j^0$ and the applied strain fields ϵ_i^0 by $\delta\epsilon_i^0$. Now consider the walled material with a geometry described by the characteristic function

$$\chi_w(\mathbf{x}) = \prod_{k=1}^3 (1 - H_{\epsilon'}(\mathbf{x} \cdot \mathbf{n}_k)), \quad (5-2)$$

where \mathbf{n}_1 , \mathbf{n}_2 , and \mathbf{n}_3 are the three orthogonal unit vectors

$$\mathbf{n}_1 = \begin{pmatrix} 1 \\ 0 \\ 0 \end{pmatrix}, \quad \mathbf{n}_2 = \begin{pmatrix} 0 \\ 1 \\ 0 \end{pmatrix}, \quad \mathbf{n}_3 = \begin{pmatrix} 0 \\ 0 \\ 1 \end{pmatrix}, \quad (5-3)$$

and ϵ' is a small parameter that gives the thickness of the walls. Inside the walls, where $\chi_w(\mathbf{x}) = 0$, we put an isotropic composite of phase 1 and phase 2, mixed in the proportions f and $1 - f$ with isotropic effective elasticity tensor $\mathbf{C}(\kappa_0, \mu_0)$, where κ_0 is the effective bulk modulus and μ_0 is the effective shear modulus, which are assumed to have finite limits as $\delta \rightarrow \infty$. (The isotropic composite could consist of islands of void surrounded by phase 1.) Outside the walls, where $\chi_w(\mathbf{x}) = 1$, we put the material that has effective compliance tensor

$$[\mathbf{C}_*^1]^{-1} = [\mathbf{C}_*(\sigma_1^0, \sigma_2^0, \sigma_3^0, \sigma_4^0, \epsilon_1^0, \epsilon_2^0)]^{-1}.$$

Let \mathbf{C}'_* be the effective tensor of the composite. We have the variational principle

$$\begin{aligned} & \sum_{i=1}^2 (\epsilon_i^0 + \delta\epsilon_i^0) : \mathbf{C}'_*(\epsilon_i^0 + \delta\epsilon_i^0) + \sum_{j=1}^4 (\sigma_j^0 + \delta\sigma_j^0) : (\mathbf{C}'_*)^{-1}(\sigma_j^0 + \delta\sigma_j^0) \\ &= \min_{\epsilon_1, \epsilon_2, \epsilon_3, \epsilon_4, \sigma_1, \sigma_2} \left\langle \sum_{i=1}^2 \underline{\epsilon}_i(\mathbf{x}) : [\chi_w(\mathbf{x})\mathbf{C}'_* + (1 - \chi_w(\mathbf{x}))\mathbf{C}(\kappa_0, \mu_0)]\underline{\epsilon}_i(\mathbf{x}) \right. \\ & \quad \left. + \sum_{j=1}^4 \underline{\sigma}_j(\mathbf{x}) : [\chi_w(\mathbf{x})\mathbf{C}'_* + (1 - \chi_w(\mathbf{x}))\mathbf{C}(\kappa_0, \mu_0)]^{-1}\underline{\sigma}_j(\mathbf{x}) \right\rangle, \quad (5-4) \end{aligned}$$

where the minimum is over fields subject to the appropriate average values and differential constraints. We choose constant trial stress fields

$$\underline{\sigma}_j(\mathbf{x}) = \sigma_j^0 + \delta\sigma_j^0, \quad j = 1, 2, 3, 4, \quad (5-5)$$

and trial strain fields

$$\underline{\epsilon}_i(\mathbf{x}) = \epsilon_i^0 + \delta\underline{\epsilon}_i(\mathbf{x}), \quad i = 1, 2, \quad (5-6)$$

where $\delta\underline{\epsilon}_i(\mathbf{x})$ has average value $\delta\epsilon_i^0$ and is concentrated in the walls. Specifically,

if $\{\delta\epsilon_i^0\}_{k\ell}$ denote the matrix elements of $\delta\epsilon_i^0$, and letting

$$\begin{aligned}\delta\epsilon_i^1 &= \begin{pmatrix} \{\delta\epsilon_i^0\}_{11} & \{\delta\epsilon_i^0\}_{12} & 0 \\ \{\delta\epsilon_i^0\}_{21} & 0 & 0 \\ 0 & 0 & 0 \end{pmatrix}, \\ \delta\epsilon_j^2 &= \begin{pmatrix} 0 & 0 & 0 \\ 0 & \{\delta\epsilon_i^0\}_{22} & \{\delta\epsilon_i^0\}_{23} \\ 0 & \{\delta\epsilon_i^0\}_{32} & 0 \end{pmatrix}, \\ \delta\epsilon_j^3 &= \begin{pmatrix} 0 & 0 & \{\delta\epsilon_i^0\}_{13} \\ 0 & 0 & 0 \\ \{\delta\epsilon_i^0\}_{31} & 0 & \{\delta\epsilon_i^0\}_{33} \end{pmatrix},\end{aligned}\tag{5-7}$$

then we choose

$$\delta\underline{\epsilon}_i(\mathbf{x}) = \sum_{k=1}^3 \delta\epsilon_i^k H_{\epsilon'}(\mathbf{x} \cdot \mathbf{n}_k) / \epsilon',\tag{5-8}$$

which has the required average value $\delta\sigma_j^0$ and satisfies the differential constraints appropriate to a strain field because $\delta\epsilon_i^k = \mathbf{a}_{i,k} \mathbf{n}_k^T + \mathbf{n}_k \mathbf{a}_{i,k}^T$ for some vector $\mathbf{a}_{i,k}$.

Hence, there exist constants α and β such that for sufficiently small ϵ' and for sufficiently small variations $\delta\sigma_j^0$ and $\delta\epsilon_i^0$ in the applied fields, we have

$$\begin{aligned}\left\langle \sum_{i=1}^2 \underline{\epsilon}_i(\mathbf{x}) : [\chi_w(\mathbf{x}) \mathbf{C}_*^1 + (1 - \chi_w(\mathbf{x})) \mathbf{C}(\kappa_0, \mu_0)] \underline{\epsilon}_i(\mathbf{x}) \right. \\ \left. + \sum_{j=1}^4 \underline{\sigma}_j(\mathbf{x}) : [\chi_w(\mathbf{x}) \mathbf{C}_*^1 + (1 - \chi_w(\mathbf{x})) \mathbf{C}(\kappa_0, \mu_0)]^{-1} \underline{\sigma}_j(\mathbf{x}) \right\rangle \\ \leq W_f^2(\sigma_1^0, \sigma_2^0, \sigma_3^0, \sigma_4^0, \epsilon_1^0, \epsilon_2^0) + \alpha\epsilon' + \beta K / \epsilon',\end{aligned}\tag{5-9}$$

where K represents the norm

$$K = \sqrt{\sum_{i=1}^2 \delta\epsilon_i^0 : \delta\epsilon_i^0 + \sum_{j=1}^4 \delta\sigma_j^0 : \delta\sigma_j^0}\tag{5-10}$$

of the field variations. Choosing $\epsilon' = \sqrt{\beta K / \alpha}$ to minimize the right-hand side of (5-9), we obtain

$$\begin{aligned}W_f^2(\sigma_1^0 + \delta\sigma_1^0, \sigma_2^0 + \delta\sigma_2^0, \sigma_3^0 + \delta\sigma_3^0, \sigma_4^0 + \delta\sigma_4^0, \epsilon_1^0 + \delta\epsilon_1^0, \epsilon_2^0 + \delta\epsilon_2^0) \\ \leq W_f^2(\sigma_1^0, \sigma_2^0, \sigma_3^0, \sigma_4^0, \epsilon_1^0, \epsilon_2^0) + 2\sqrt{\alpha\beta K}.\end{aligned}\tag{5-11}$$

Clearly the right-hand side approaches $W_f^2(\sigma_1^0, \sigma_2^0, \sigma_3^0, \sigma_4^0, \epsilon_1^0, \epsilon_2^0)$ as $K \rightarrow 0$. On the other hand, by repeating the same argument with the roles of

$$W_f^2(\sigma_1^0, \sigma_2^0, \sigma_3^0, \sigma_4^0, \epsilon_1^0, \epsilon_2^0)$$

and

$$W_f^2(\sigma_1^0 + \delta\sigma_1^0, \sigma_2^0 + \delta\sigma_2^0, \sigma_3^0 + \delta\sigma_3^0, \sigma_4^0 + \delta\sigma_4^0, \epsilon_1^0 + \delta\epsilon_1^0, \epsilon_2^0 + \delta\epsilon_2^0)$$

reversed, and with the compliance tensor

$$[C_*(\sigma_1^0 + \delta\sigma_1^0, \sigma_2^0 + \delta\sigma_2^0, \epsilon_1^0 + \delta\epsilon_1^0, \epsilon_2^0 + \delta\epsilon_2^0, \epsilon_3^0 + \delta\epsilon_3^0, \epsilon_4^0 + \delta\epsilon_4^0)]^{-1}$$

replacing the compliance tensor

$$[C_*(\sigma_1^0, \sigma_2^0, \sigma_3^0, \sigma_4^0, \epsilon_1^0, \epsilon_2^0)]^{-1},$$

we deduce that

$$\begin{aligned} & W_f^2(\sigma_1^0, \sigma_2^0, \sigma_3^0, \sigma_4^0, \epsilon_1^0, \epsilon_2^0) \\ & \leq W_f^2(\sigma_1^0 + \delta\sigma_1^0, \sigma_2^0 + \delta\sigma_2^0, \sigma_3^0 + \delta\sigma_3^0, \sigma_4^0 + \delta\sigma_4^0, \epsilon_1^0 + \delta\epsilon_1^0, \epsilon_2^0 + \delta\epsilon_2^0) \\ & \quad + 2\sqrt{\alpha\beta K}. \end{aligned} \quad (5-12)$$

This, together with (5-11), establishes the continuity of $W_f^2(\sigma_1^0, \sigma_2^0, \sigma_3^0, \sigma_4^0, \epsilon_1^0, \epsilon_2^0)$. The continuity of the other energy functions follows by the same argument.

6. Conclusion

We have established the following theorems.

Theorem 6.1. *Consider composites in three dimensions of two materials with positive definite elasticity tensors C_1 and $C_2 = \delta C_0$ mixed in proportions f and $1 - f$. Let the seven energy functions W_f^k , for $k = 0, 1, \dots, 6$, that characterize the set GU_f (with $U = (C_1, \delta C_0)$) of possible elastic tensors be defined by (2-1). These energy functions involve a set of applied strains ϵ_i^0 and applied stresses σ_j^0 meeting the orthogonality condition (2-2). The energy function W_f^6 is given by*

$$W_f^6(\epsilon_1^0, \epsilon_2^0, \epsilon_3^0, \epsilon_4^0, \epsilon_5^0, \epsilon_6^0) = \sum_{i=1}^6 \epsilon_i^0 : C_f^A(\epsilon_1^0, \epsilon_2^0, \epsilon_3^0, \epsilon_4^0, \epsilon_5^0, \epsilon_6^0) \epsilon_i^0 \quad (6-1)$$

(as established by Avellaneda [1987]), where $C_f^A(\epsilon_1^0, \epsilon_2^0, \epsilon_3^0, \epsilon_4^0, \epsilon_5^0, \epsilon_6^0)$ is the effective elasticity tensor of an Avellaneda material that is a sequentially layered laminate with the minimum value of the sum of elastic energies

$$\sum_{i=1}^6 \epsilon_i^0 : C_* \epsilon_i^0. \quad (6-2)$$

Again some of the applied stresses σ_j^0 or applied strains ϵ_i^0 could be zero. Additionally, we have

$$\begin{aligned} \lim_{\delta \rightarrow \infty} W_f^0(\sigma_1^0, \sigma_2^0, \sigma_3^0, \sigma_4^0, \sigma_5^0, \sigma_6^0) &= 0, \\ \lim_{\delta \rightarrow \infty} W_f^1(\sigma_1^0, \sigma_2^0, \sigma_3^0, \sigma_4^0, \sigma_5^0, \epsilon_1^0) &= \epsilon_1^0 : [C_f^A(0, 0, 0, 0, 0, \epsilon_1^0)]\epsilon_1^0, \\ \lim_{\delta \rightarrow \infty} W_f^2(\sigma_1^0, \sigma_2^0, \sigma_3^0, \sigma_4^0, \epsilon_1^0, \epsilon_2^0) &= \sum_{i=1}^2 \epsilon_i^0 : [C_f^A(0, 0, 0, 0, \epsilon_1^0, \epsilon_2^0)]\epsilon_i^0, \\ \lim_{\delta \rightarrow \infty} W_f^3(\sigma_1^0, \sigma_2^0, \sigma_3^0, \epsilon_1^0, \epsilon_2^0, \epsilon_3^0) &= \sum_{i=1}^3 \epsilon_i^0 : [C_f^A(0, 0, 0, \epsilon_1^0, \epsilon_2^0, \epsilon_3^0)]\epsilon_i^0, \end{aligned} \quad (6-3)$$

for all combinations of applied stresses σ_j^0 and applied strains ϵ_i^0 . In the case that $\det(\sigma_1^0) = 0$, we have

$$\lim_{\delta \rightarrow \infty} W_f^5(\sigma_1^0, \epsilon_1^0, \epsilon_2^0, \epsilon_3^0, \epsilon_4^0, \epsilon_5^0) = \sum_{i=1}^5 \epsilon_i^0 : [C_f^A(0, \epsilon_1^0, \epsilon_2^0, \epsilon_3^0, \epsilon_4^0, \epsilon_5^0)]\epsilon_i^0, \quad (6-4)$$

while, when $f(t) = \det(\sigma_1^0 + t\sigma_2^0)$ has at least two roots (the condition for which is given by (4-4)),

$$\lim_{\delta \rightarrow \infty} W_f^4(\sigma_1^0, \sigma_2^0, \epsilon_1^0, \epsilon_2^0, \epsilon_3^0, \epsilon_4^0) = \sum_{i=1}^4 \epsilon_i^0 : [C_f^A(0, 0, \epsilon_1^0, \epsilon_2^0, \epsilon_3^0, \epsilon_4^0)]\epsilon_i^0. \quad (6-5)$$

Theorem 6.2. For 2-dimensional composites, the four energy functions W_f^k , for $k = 0, 1, 2, 3$, are defined by (2-5), and these characterize the set GU_f , with $U = (C_1, \delta C_0)$, of possible elastic tensors C_* of composites of two phases with positive definite elasticity tensors C_1 and $C_2 = \delta C_0$. These energy functions involve a set of applied strains ϵ_i^0 and applied stresses σ_j^0 meeting the orthogonality condition (2-2). The energy function W_f^3 is given by

$$W_f^3(\epsilon_1^0, \epsilon_2^0, \epsilon_3^0) = \sum_{i=1}^3 \epsilon_i^0 : C_f^A(\epsilon_1^0, \epsilon_2^0, \epsilon_3^0)\epsilon_i^0 \quad (6-6)$$

(as proved by Avellaneda [1987]), where $C_f^A(\epsilon_1^0, \epsilon_2^0, \epsilon_3^0)$ is the effective elasticity tensor of an Avellaneda material that is a sequentially layered laminate with the minimum value of the sum of elastic energies

$$\sum_{j=1}^3 \epsilon_j^0 : C_* \epsilon_j^0. \quad (6-7)$$

We also have the trivial result that

$$\lim_{\delta \rightarrow \infty} W_f^0(\sigma_1^0, \sigma_2^0, \sigma_3^0) = 0. \quad (6-8)$$

When $\det \sigma_1^0 = 0$, we have

$$\lim_{\delta \rightarrow \infty} W_f^2(\sigma_1^0, \epsilon_1^0, \epsilon_2^0) = \sum_{i=1}^2 \epsilon_i^0 : [C_f^A(0, \epsilon_1^0, \epsilon_2^0)] \epsilon_i^0, \quad (6-9)$$

while when $f(t) = \det(\sigma_1^0 + t\sigma_2^0)$ has exactly two roots (the condition for which is given by (4-2)),

$$\lim_{\delta \rightarrow \infty} W_f^1(\sigma_1^0, \sigma_2^0, \epsilon_1^0) = \epsilon_1^0 : [C_f^A(0, 0, \epsilon_1^0)] \epsilon_1^0. \quad (6-10)$$

These theorems, and the accompanying microstructures, help define what sort of elastic behaviors are theoretically possible in 2- and 3-dimensional materials consisting of a very stiff phase and an elastic phase (possibly anisotropic, but with fixed orientation). They should serve as benchmarks for the construction of more realistic microstructures that can be manufactured. We have found the minimum over all microstructures of various sums of energies and complementary energies.

It remains an open problem to find expressions for the energy functions in the cases not covered by these theorems. Notice that for 3-dimensional composites the function W_f^5 is only determined when the special condition $\det(\sigma_1^0) = 0$ is satisfied exactly. Similarly, for 2-dimensional composites the function W_f^2 is only determined when the special condition $\det \sigma_1^0 = 0$ is satisfied exactly. Thus these functions are only known on a set of zero measure.

Even for an isotropic composite with a bulk modulus κ_* and a shear modulus μ_* , the set of all possible pairs (κ_*, μ_*) is still not completely characterized either in the limit $\delta \rightarrow \infty$. In these limits the bounds of Berryman and Milton [1988] and Cherkaev and Gibiansky [1993] decouple and provide no extra information beyond that provided by the Hashin–Shtrikman–Hill bounds [Hashin and Shtrikman 1963; Hashin 1965; Hill 1963; 1964]. While the results of this paper show that in the limit $\delta \rightarrow \infty$ one can obtain 3-dimensional structures attaining the Hashin–Shtrikman–Hill lower bound on κ_* , while having $\mu_* = \infty$, it is not clear what the minimum value for μ_* is, given that $\kappa_* = \infty$, nor is it clear in two dimensions what the minimum value of κ_* is when $\mu_* = \infty$.

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Reducible and irreducible forms of stabilised gradient elasticity in dynamics	1
Harm Askes and Inna M. Gitman	
Dating Hypatia's birth: a probabilistic model	19
Canio Benedetto, Stefano Isola and Lucio Russo	
On the possible effective elasticity tensors of 2-dimensional and 3-dimensional printed materials	41
Graeme W. Milton, Marc Briane and Davit Harutyunyan	
Towards a complete characterization of the effective elasticity tensors of mixtures of an elastic phase and an almost rigid phase	95
Graeme W. Milton, Davit Harutyunyan and Marc Briane	

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